

Artigos

Hydraulic simulation in fractured media using 1D equivalent pipes - FratCond tool module II

Simulação hidráulica em meios fraturados utilizando condutos unidimensionais equivalentes – módulo II da ferramenta FratCond

Alan Reis1; José Eduardo Alamy Filho2 🖂

¹ Universidade de São Paulo (USP), São Paulo, SP ² Universidade Federal de Uberlândia (UFU), Uberlândia, MG

alanreis.ar@gmail.com, zealamy@yahoo.com.br

Keywords:

MATLAB.

Fractured aquifers.

1D equivalent pipes.

Numerical modelling.

Discrete fracture network (DFN).

Abstract

Different conceptual models for hydraulic simulation in fractured media are present in the literature, such as the equivalent porous model, the double porosity model and the use of discrete fracture networks. Considering this last approach, some studies have been successfully evaluated the possibility of summarizing the geometry of fractures as a set of conduits or channels. In this way, it is possible to verify the fluid flow in these discontinuities in a simplified way, reducing the computational effort of the simulations, when compared to the traditional approaches. In this sense, this work aims to present the implementation of a computational tool intended to perform steady flow simulations in fractured media, using equivalent one-dimensional pipes. From a discrete fractures network, previously generated, these conduits are made by connecting the fractures intersections with their respective centers, which become reference nodes. With this geometry, it is possible to estimate the total head at these nodes and the flow rates in the fractures, using mass balance equations. The medium hydraulic properties, such as transmissivity, fracture aperture and length of the conduit created, are taken into account. In this way, it allows the proper evaluation of this phenomenon, both in terms of the flow rates and for the possible evaluation of the transport of pollutants in these media.

ÅGUAS SUBTERR

Palavras-chave:

Meios fraturados. Redes de fraturas discretas. Condutos unidimensionais equivalentes. Modelagem numérica. MATLAB.

Revisado por pares. Recebido em: 07/08/2019. Aprovado em: 20/02/2020.

Resumo

Diferentes modelos conceituais para a simulação hidráulica em meios fraturados estão presentes na literatura, dentre os quais se destacam o modelo poroso equivalente, o modelo de dupla porosidade e o uso das redes de fraturas discretas. Considerando esta última abordagem, foram realizados alguns estudos avaliando de forma bem-sucedida a possibilidade de se resumir a geometria das fraturas como um conjunto de condutos ou canais. Desta forma, é possível verificar o trânsito de fluido nestas descontinuidades de maneira simplificada, reduzindo o esforço computacional das simulações, quando comparado às abordagens tradicionais. Neste sentido, este trabalho visa apresentar a implementação de uma ferramenta computacional destinada a realização de simulação de escoamentos permanentes em meios fraturados, utilizando condutos unidimensionais equivalentes. Tais condutos são construídos a partir de uma rede de fraturas discretas, previamente gerada, ligando as interseções das fraturas com seus respectivos centros, que se transformam em nós de referência. A partir do uso de equações de balanço de massa, é possível estimar as cargas hidráulicas nestes nós e as vazões que transitam no interior das fraturas. As propriedades hidráulicas do meio, tais como a transmissividade, a abertura da fratura e o comprimento do conduto criado, são devidamente levadas em conta. Desta forma, permite-se a avaliação adequada deste fenômeno, tanto em termos do escoamento propriamente dito, como para uma possível avaliação de transporte de poluentes nestes meios.

DOI: http:/dx.doi.org/10.14295/ras.v34i1.29591

1. INTRODUCTION

Hydraulic simulation in fractured media is a complex modeling, since it deals with formations that have anisotropic characteristics. Volcanic and metamorphic rocks are examples of materials that make up fractured formations, which commonly have two distinct regions, in terms of fluid movement.. The fractures (or joints), that are planes along which stress has caused partial loss of cohesion in the rock, representing a plane of weakness (discontinuity) in the rock (SINGHAL and GUPTHA, 2010). These planes are quite permeable, facilitating flow occurrence. Meanwhile, the rocky matrix, formed by the cohesive material, presents low or no permeability, hindering the passage of fluids.

Considering these features, some conceptual models for hydraulic modeling are present in the literature, and can be differentiated into three groups: equivalent porous medium model, double porosity model (BARENBLATT *et al.*, 1960; STRELTSOVA, 1976; GRINGARTEN, 1982) and discrete fractures network (SNOW, 1965). Variations in these models are noted, considering the way the fracutres are represented, the scale of interest for modeling and the model purpose, in relation to flow evaluation and solute transport.

Considering the third model category, some studies carried out by different researchers (CACAS et al., 1990a; SEGAN and KA-RASAKI, 1993; DERSHOWITZ, 1996; UBERTOSI et al., 2007; XU et al., 2014) noted the potential of good flow modeling in fractured media, using 1D pipes and channels. This intention aims to reduce computational effort involved in this simulation, considering that such phenomena generally involve complex differential equations, which can be simplified, using flow balance equations in the pipe network, formed from the fractures. In addition, it is known that in fracture planes, only a few regions are effectively wetted, while others remain dry, due to the local variations in aperture and roughness in fracture surface, causing the channeling phenomenon, reported in several studies involving fractured media. According to Ubertosi et al. (2007), in general, only 30% of the fracture surface area is covered by flow, forming preferential paths.

The way to obtain conduits and channels representing discrete fractures is visualized in literature in two modes: from a single representative path, which interconnects the fracture center (considering the different geometric shapes used in its representation) to the intersection central points with the neighboring discontinuities (CACAS *et al.*, 1990a; CACAS *et al.*, 1990b; MORENO e and NERETNIEKS, 1993; DERSHOWITZ, 1996) or by building a conduit mesh in the fracture plane, with variations in the properties that influence fluid transit (UBERTOSI *et al.*, 2007; BODIN *et al.*, 2007). In these studies, both permanent flow simulation and solute transport are presented with satisfactory results, when calibrated and compared with field data.

Considering this context, this study aims to present the aspects involved in the development of a computational tool for permanent flow simulation in fractured media, considering the use of equivalent 1D equivalent pipes. This tool corresponds to FratCond tool module 2, being responsible for the 1D pipe network generation and for the hydraulic heads and flow rates determination, considering contour conditions inserted by the user. The geometry is based on stochastic fracture generation, obtained from module 1 (REIS, ALAMY FILHO, 2018). After the simulation, the tool has graphs and worksheets available for results visualization and analysis.

2. METHODOLOGY

FratCond module 2 was developed using MATLAB program ming language, being coupled to module 1, as presented by Reis e Alamy Filho (2018). In order to execute module 2, it is necessary to execute the first one, since all model geometry depends on the results obtained in module 1. This generation is based on the main probability density functions (PDFs) defined for the variables that characterize the fractures: length (lognormal distribution), aperture (lognormal distribution), orientation (dip and direction diving; Fisher distribution) and location (fracture center; uniform distribution). From statistical values of these variables (mean and standard deviation), these PDFs can be used to generate fracture networks with similar characteristics to the original, allowing the hydraulic modeling of such media (REIS, ALAMY FILHO, 2018). More details on stochastic approaches related to fractured media can be found in Cacas et al. (1990a), Cacas et al. (1990b), who modeled the Fanay-Augères (France) fractured system using this concept; and also Jing (2003), Dershowitz, Pointe e Doe (2004) e Chilès (2005), where complete reviews about this method can be found.

The module 2 is responsible only for the hydraulic simulation itself. The hydraulic simulation proposed by Fratcond tool is based on the reduction of the discrete fractures network, stochastically generated, to a network of equivalent 1D pipes, as proposed by Cacas *et al.* (1990a), Cacas *et al.* (1990b), Dershowitz (1996) and Outters *et al.* (2000).

These pipes are generated, in parts, from the connection between the intersection midpoint of two fractures and their respective centers. Figure 1 illustrates this generation process, considering fractures represented by Baecher model (circular disks). Each pipe (e.g. T_1, T_2, T_3 in Figure 1) is divided in two parts (e.g. T_{1a} and T_{1b} in Figure 1), once each part occurs at the surface of a different fracture, that form the intersection analyzed, with a corresponding change in its orientation, in terms of direction and dip. The repetition of this point interconnection, for all fractures connected to each other, allows obtaining the pipe network representative of the fractured system under study (CACAS et al., 1990a; CACAS et al., 1990b; DERSHOWITZ, 1996; OUTTERS et al., 2000).

It is important to note that the pipe reference nodes are always the fractures center points (black dots in Figure 1), where the representative hydraulic load values will be determined for each fracture. In addition, different pipes will meet in these points, demanding a flow rate balance. The intersection midpoints (red dots in Figure 1) are only basis for pipe network tracing, but do not configure a node itself.

Every fracture that presents intersection with a second one has its representative pipe traced. Isolated fractures, which do not intercept with others, will not participate of pipe network, since there is a dependence on the intersection existence for the pipe tracing.



For FratCond module 2 execution, two additional information must be provided by the user as input data, which enable hydraulic equations resolution. The first data are the hydraulic head values in the volume simulation limits, which function as a contour condition for the hydraulic simulation. At least one of the four faces that are perpendicular to the xy plane (Figure 2), forming the cubic or prismatic volume considered in the

simulation, need hydraulic head information. In this way, a regional hydraulic gradient can be obtained, indicating flow trend direction inside the simulated formation. Such contour conditions can be acquired in the field, using water level of observation wells, in limits close to the volume considered for the simulation; or even estimated by the user, from observation wells data, that are not necessarily in the perimeters of that volume.



Figure 2 - Simulation volume faces: reference used to indicate boundary conditions

The second input data required is conductance. Conductance is a coefficient of proportionality between flow and hydraulic gradient, in each one-dimensional equivalent pipe (Moreno and Neretnieks, 1993; Dershowitz, 1996). Two approaches are possible: considering mathematical relationships based on fracture features, such as transmissivity and fracture opening (Dershowitz, 1996); or from the use of statistical values and a lognormal distribution. In the last case, statistical values are obtained from estimation or from simulated model's calibration with field data (considering the relationship between flow, conductance and hydraulic gradients), once conductance is not a physical parameter, directly determined in the fractured media.

Prior to the hydraulic calculation itself, some actions are still necessary for successful simulation. The first action is to separate the equivalent 1D pipes group with more connections, in relation to secondary groups, with less extension or isolated fractures. It is observed in real fractured media that some fractures groups are isolated and do not contribute effectively to water flow. The same happens in the simulations executed in FratCond tool, where it is impossible to consider all the fractures generated in module 1 for hydraulic simulation, which would require multiple contour conditions and could reduce fidelity, when comparing model to reality. Thus, the chosen option was to keep only the longest pipe group, which have a greater chance to traverse the whole simulation volume. This isolation was performed using graph function, available in latest MATLAB versions (later than R2015b). This function analyzes a two columns matrix, considering elements in the same line as nodes. Then, the function informs which paths, formed by these nodes, are connected to each other, through a connection identifier. The identifier with the highest number of

repetitions represents the longest pipe group, among all other paths previously obtained.

Another necessary action is to carry out contour conditions, from volume simulation faces to the most extreme pipe network nodes. This transport needs to be done, once the pipe network limits do not always match the volume simulation limits. Thus, the contour conditions in pipe network will be approximated from the values inserted by the user at the volume simulation faces This approximation was made using linear interpolation, considering gradients formed along the x and y axes, which are limited by the volume faces that have the insertion possibility of initial hydraulic heads by the user. Figure 3 illustrates the variables involved in this transport along the x-axis of the simulation volume. Along the y-axis, the process is analogous. The respective meanings of each term are:

- $CH_{esq}(CH_{frontal})$: total head considered in left face (frontal face) of simulation volume;
- CH_{dir} (CH_{fundo}): total head considered in right face (back face) of simulation volume;
- x_{min} (y_{min}): fracture center coordinate, of minimum value, in x axis (in y axis);
- $x_{max}(y_{max})$: fracture center coordinate, of maximum value, in x axis (in y axis);
- $x_{\max volume} (y_{\max volume})$: edge size of simulation volume along x axis (along y axis);
- CH_{xmin} (CH_{ymin}): total head imposed at the extreme nodes of pipe network, with minimum coordinate value, in x axis (in y axis);
- CH_{xmax} (CH_{ymax}): total head imposed at the extreme nodes of pipe network, with maximum coordinate value, in x axis (in y axis);



After completing these actions, the next step is to solve the system of mass balance equations (Equations 1 and 2). Here, we seek to calculate flow rates of each fracture (and in each 1D pipe), in a steady state, as well as total head values in each pipe system nodes, which corresponds to fractures centers. Conductance and pipe lengths are constants and defined previously to this resolution. It is important to know that inflows and outflows are not considered in these equations.

$$\begin{cases} Q_{ij} = C_{ij} \times I_{ij} & (in each pipe stretch) \\ \sum Q = 0 & (in each node) \end{cases}$$
(1)

 $\left(\sum_{i}Q=0\right)$ where:

 Q_{ii} : flow rate in 1D equivalent pipe, formed between fractures i and j (m^3/s);

 C_{ij} : conductance between fractures *i* and *j* (m³/s);

 I_{ij} : hydraulic gradient between fractures *i* and *j* (m/m), given by total head difference (m), divided by pipe length (m).

In addition, the boundary conditions transported to extreme nodes of pipe system must be considered. In this way, total head in the extreme node is equated to the value calculated in transport, as previously described. At least 1 and up to 4 additional equalities can be inserted, depending on the number of boundary conditions inserted in the simulated model.

This equation system is arranged in $A \times X = B$ matrix form, as genererally presented in Equation 3. Matrix A presents conductances and pipe lengths, with positive sign for flow rates arriving and negative sign for flow rates leaving the analyzed node. In the last matrix lines, values of 1 or 0 are inserted, considering boundary conditions inserted in the model. Matrix X list all total head values that will be calculated. Matrix B presents null values for most of the lines, except for boundary condition lines, where the total head values defined in the transfer to the extreme nodes are placed.

г р ч

$$\begin{bmatrix} \frac{c_{12}}{L_{12}} + \frac{c_{13}}{L_{13}} + \frac{c_{1n}}{L_{1n}} & -\frac{c_{12}}{L_{12}} & -\frac{c_{13}}{L_{13}} & \dots & -\frac{c_{1n}}{L_{1n}} \end{bmatrix} \times \begin{bmatrix} n_1 \\ h_2 \\ h_3 \\ \vdots \\ h_n \end{bmatrix} = \begin{bmatrix} 0 \\ 100 \end{bmatrix}$$

To solve this system, the LSOR method (Least Squares with OR Factorization), developed by Paige and Sounders (1982) and available in MATLAB, was used. A maximum residual error of $10^{-4} \mbox{ and a maximum iterations number of } 10^6 \mbox{ was consid-}$ ered. Such values were considered sufficient to obtain good results with the use of this numerical method, without the occurrence of numerical instabilities. With the system solved, the values of total head are obtained in each of pipe network nodes. For consequence, we can calculate values of hydraulic gradients and flow rates, replacing such results in the previously presented equations.

3. RESULTS AND DISCUSSIONS

The results below show the products obtained with the implementation of the method presented previously. FratCond tool user can exploit all these products. All results shown here take into account the fractures generated stochastically by FratCond module 1, from a proposed example, as presented by Reis e Alamy Filho (2018).

As discussed previously, additional input data are required for the proposed hydraulic simulation execution. The user can input this data using the module 2 graphical interface (in Portuguese), illustrated in Figure 4. The blank spaces should be filled with the total head in simulation volume faces and the user need to choose the way conductance will be determined, in the model to be simulated. After this, the simulation can be performed, from the activation of the "Simulação Hidráulica" (Hydraulic Simulation) button.

Figure 4 - FratCond tool module 2 graphic interface



(3)

After a few seconds, the user receives a confirmation of successfully routines execution. From this moment, the user can explore simulation results, using the section "Explorar resultados" (Exploring results), which has a list of 6 graphs and 1 spreadsheet, which can be exported. To do this, the user choose the result of interest and then click on the "Ok" button, so that it is displayed.

Figure 5 presents the first result available in FratCond module 2, with the 1D pipes network, generated from the fractures obtained stochastically, in the hypothetical example presented by Reis e Alamy Filho (2018). The image shows the fracture identifiers (numbers in red) next to their centers (pink dots), as well as the points of intersection between the fractures (black dots) and their respective identification numbers. The blue lines represent the equivalent 1D pipes. We noted, as expected, the sequence of fracture center-intersection-fracture center interconnection respected, following the methodology used. We can also see that not all fractures are connected to each other, there are pipe groups isolated inside the simulation volume. Secondary pipe networks can be seen near the left side of the volume (between nodes 8 and 25) and the right side (between nodes 15 and 55). The pipe group with the highest number of connections occurs in the lower region of the simulated medium, bounded by nodes 11 and 27. This section will be isolated and hydraulically simulated.



Figure 5 - 1D equivalent pipes generated using FratCond module 2

Source: Authors (2020)

Figure 6 shows the graph available for conductance results analysis, considering all equivalent pipes generated by the model or only the pipes belonging to the longest group. This parameter works as proportionality coefficient between flow and hydraulic gradient. In this hypothetical example, the conductance was defined from fractures properties, according to the methodology of Dershowitz (1996). Results can be checked from the displayed color scale. Light green tones indicate lower values, while dark blue tones indicate higher values. These values are changed at the pipe nodes, indicated by the pink points (fracture centers). The identifiers now shown are pipes identities.

Observing the results obtained in this proposed hypothetical example, we note a high variation of this parameter, when determined from the fracture features. Among them, the one that cause the greatest influence on conductance values are the fracture aperture, indicating more or less ease for flow; and how long is the intersection between the fractures that generate each pipe, which is also indicative of ease for fluid passage.

The following figures show direct results of the hydraulic simulation itself. Figure 7 presents results obtained for hydraulic gradients, while Figure 8 presents flow values obtained for each pipe, allowing a preview of the water volume that transits between the fractures. In both graphs, we can see the highest values occur in pipes 14 and 15, which are highlighted by the blue color. This result is consistent, since both quantities are directly proportional. In addition, for this hypothetical example, boundary conditions entered create a larger gradient in the xaxis direction, when compared with y-axis direction. Considering that pipes 14 and 15 are practically parallel to the axis of higher hydraulic gradient, there will be a tendency of greater flow through these pipes.



Figure 6 - Conductance values (m³/s) obtained in the simulated model: (a) for all pipes and (b) for the longest pipe group

Source: Authors (2020)





Source: Authors (2020)

It is noteworthy the possibility of inconsistencies in the displayed graphs, due to the precision entered in the numerical calculation and possible rounding performed, especially regarding the flow balance in each pipe node. As an example, looking at pipe 16 in Figure 8, despite its continuity with pipes 14 and 15, this tube had a slightly different flow rate, changing its color class in the graph displayed. However, by looking at the result spreadsheet with the numerical values, it is possible to verify properly that this is an acceptable rounding problem, at the fourth decimal place.

Figure 8 - Flow rates (m³/s) calculated in proposed example



Source: Authors (2020)

Finally, the last available graph (Figure 9) shows the total head values obtained in each of the pipe network nodes (fracture centers). This graph shows color-scale circles that indicate the

respective total head results. Identifiers for fracture centers (red numbers) and pipe stretches (black digits) are also dis-

played in this view, and the numbers surrounded by a rectangle indicate pipe nodes that have received the boundary conditions, i.e., they are extreme nodes, closest to simulation volume faces. For this hypothetical example, as boundary conditions were imposed in all possible simulation faces, four nodes are highlighted, with nodes 27 and 11 receiving right and left faces conditions, while nodes 20 and 14 receive from the bottom and front faces conditions, respectively. We also note that the total head values obtained respect flow tendency imposed by the boundary conditions initially inserted in the model. Thus, there is a reduction in total head from right to left and bottom to front, with the largest contributions, in terms of flow rates, appearing in pipes that are parallel to the x-axis.





Source: Authors (2020)

As a last form of results analysis, the tool has an exportable spreadsheet available (Figure 10), which gathers the main data obtained in the hydraulic simulation, in numerical form. Complementing module 1 spreadsheet, module 2 provides two tabs, indicated by "Dados dos trechos" (Pipe Data) and "Caminho mais longo" (Longest Pipe Group Data).

The first tab presents data used to determine conductance, such as pipe length and width, the average pipe aperture, the average transmissivity and the respective conductance value obtained. For a complete understanding of the relationship between these parameters, it is recommended to consult the study of Dershowitz (1996). A pipe identifier, followed by the indication of the respective fractures that form it, orders all these results. The second tab presents the results of the hydraulic simulation itself, for the longest pipe group. At the top, data concerning the entered and transported boundary conditions, the numerical resolution conditions of the proposed system of equations are displayed. At the bottom, for each pipe, it is listed the following data: pipe length, conductance, total head at the beginning and end of the pipe, the hydraulic gradient and the respective flow rate.

А	B	C	D	E	F	G	Н		J	ĸ	
			FratCond - R	esultados da s	simulação hidı	ráulica - Dados	dos trechos				
					-						
	Data e hora de ge	ração da geometria:		18-01-18	15-04-16						
Data e hora da simulação hidráulica:				18-01-18	-16-03-03						
[Data e hora de expo	rtação dos resultado	s:	18-01-18	-16-11-12						
	Volume de s	imulação (m³):		125		Núm	nero de fraturas gera	adas:	57		
						Núme	ro de interseções ol	btidas:	40		
ID	fratura i	fratura j	L (m)	W (m)	a (m)	T (m²/s)	C (m³/s)				
1	1	37	7.0375	2.3167	0.0233	0.0022	0.0050				
2	1	43	7.8150	2.0374	0.0216	0.0019	0.0038				
3	1	46	7.2263	2.0194	0.0225	0.0020	0.0041				
4	1	57	7.0578	1.9259	0.0230	0.0021	0.0041				
5	2	36	2.5898	2.0478	0.0134	0.0007	0.0015				
6	2	40	1.5318	2.4292	0.0158	0.0010	0.0024				
7	2	56	2.1422	1.0613	0.0201	0.0016	0.0017				
8	3	26	5.1807	1.5425	0.0230	0.0021	0.0033				
9	3	30	3.6726	1.0690	0.0278	0.0031	0.0033				
10	3	49	4.6999	1.9778	0.0231	0.0021	0.0042				
11	3	50	4.1685	2.0347	0.0244	0.0024	0.0048				
12	8	34	6.7783	2.0192	0.0133	0.0007	0.0014				
13	9	37	5.4409	2.0407	0.0276	0.0031	0.0062				
Dado	os das fraturas 🛛 D	ados das interseções	Dados dos trech	Caminho mai	is longo 🛛 🕀	: •					
→ Dado	os das fraturas D	ados das interseções C	Dados dos trech	Caminho mai	is longo 🛛 🕀	i 🖣	н	1	J	к	
> Dado	os das fraturas D	ados das interseções C	Dados dos trech	E E E E E	s longo ↔ F mulação hidrá	ة المسلحة المسلح G	н o mais longo	I	J	к	
A Dado	os das fraturas D	ados das interseções C	Dados dos trech D FratCond - Re	E E E E E E E E E E E E E E E E E E E	s longo F mulação hidrá	G G iulica - Caminh	н o mais longo	Ι	J	к	
A Dado	B Data e hora de ge	ados das interseções C ração da geometria:	Dados dos trech D FratCond - Re	E E Esultados da si 18-01-18-	s longo ÷ F mulação hidrá 15-04-16	G G iulica - Caminh	н o mais longo	I	J	K	
A Dado	B B Data e hora de ge Data e hora da sin	ados das interseções C ração da geometria: nulação hidráulica:	Dados dos trech D FratCond - Re	Caminho mai E esultados da si 18-01-18- 18-01-18-	s longo F mulação hidrá 15-04-16 16-03-03	G G iulica - Caminh	н o mais longo	I	J	К	
m Dado A	B B Data e hora de ge Data e hora da sin Data e hora de expo	ados das interseções C ração da geometria: nulação hidráulica: rtação dos resultado	Dados dos trech	Caminho mai E esultados da si 18-01-18- 18-01-18- 18-01-18-	s longo (+) F mulação hidrá 15-04-16 16-03-03 16-11-12	G G G G G G G G G G G G G G G G G G G	н o mais longo	I	J	K	
Made Dade Dade Dade Dade Dade Dade Dade D	B B Data e hora de ge Data e hora da sin Data e hora de expo	ados das interseções C ração da geometria: nulação hidráulica: rtação dos resultado	Dados dos trech D FratCond - Re	Caminho mai E esultados da si 18-01-18- 18-01-18- 18-01-18-	s longo F mulação hidrá 15-04-16 16-03-03 16-11-12	∶ ∢ G iulica - Caminh	н o mais longo	1	J	K	
m Dado	B B Data e hora de ge Data e hora da sin Data e hora de expo	ados das interseções c ração da geometria: nulação hidráulica: rtação dos resultado	Dados dos trech	Caminho mai E esultados da si 18-01-18- 18-01-18- 18-01-18-	F mulação hidrá 15-04-16 16-03-03 16-11-12	i d G iulica - Caminh	H o mais longo ero de fraturas gera	ı ıdas:	57	K	
A	B B Data e hora de ge Data e hora da sin Data e hora de expo Volume de s	c c ração da geometria: nulação hidráulica: rtação dos resultado imulação (m ^a):	Dados dos trech	Caminho mai E esultados da si 18-01-18- 18-01-18- 18-01-18- 125	F F mulação hidrá 15-04-16 16-03-03 16-11-12	iulica - Caminh	H o mais longo ero de fraturas gera ro de interseções ol	ı Idas:	J 57 40	K	
> Dadc	B B Data e hora de ge Data e hora de sin Data e hora de expo Volume de s	c c ração da geometria: nulação hidráulica: rtação dos resultado imulação (m³):	Dados dos trech	Caminho mai E esultados da si 18-01-18- 18-01-18- 18-01-18- 125	F F mulação hidrá 15-04-16 16-03-03 16-11-12	iulica - Caminh	H o mais longo ero de fraturas gera ro de interseções ot terseções no camint	I Idas: otidas: no mais longo:	J 57 40 15	K	
Dado	B B Data e hora de ge Data e hora da si Data e hora da si Data e hora de expo Volume de s	c C ação da geometria: nulação hidráulica: rtação dos resultado imulação (m ³):	Dados dos trech	Caminho mai E esultados da si 18-01-18- 18-01-18- 18-01-18- 125	F F mulação hidrá 15-04-16 16-03-03 16-11-12	i I	H o mais longo ero de fraturas gera ro de interseções of terseções no camini	l Idas: titdas: no mais longo:	J 57 40 15	K	
> Dado	B B Data e hora de ge Data e hora de seo Data e hora da sin Data e hora de expo Volume de s	c c ação da geometria: nulação hidráulica: rtação dos resultado imulação (m ¹):	Dados dos trech	E E E E E E E E E E E E E E E E E E E	s longo F mulação hidrá 15-04-16 16-03-03 16-11-12	G G iulica - Caminh Múme Número de in Fraturas con	H o mais longo rero de fraturas gera ro de interseções ol terseções no camint r condição de conto	ı ıdas: ətidas: no mais longo: rno: Cargas hidrául	57 40 15	K	
> Dado	B B Data e hora de ge Data e hora de se Data e hora de se Data e hora de expo Volume de s 4úmero de iteraçõe Erro relati	c c ação da geometria: nulação hidráulica: tação dos resultado imulação (m ¹): s do cálculo hidráulic vo residual:	Dados dos trech	E E E E E E E E E E E E E E E E E E E	F F mulação hidrá 15-04-16 16-03-03 16-11-12	G G iulica - Caminh Núme Número de in Fraturas con Face do volume	H o mais longo ero de fraturas gera ro de interseções ol terseções no camini r condição de contor na face do volume	idas: otidas: no mais longo: rno: Cargas hidrául ID	57 40 15 Valor na fratura (m)	K	
Dado	B B Data e hora de ge Data e hora de sin Data e hora de sin Data e hora de sin Volume de s Volume de s Número de iteraçõe Erro relati Situação de	c c ação da geometría: nulação hidráulica: rtação dos resultado imulação (m ⁴): s do cálculo hidráulic vo residual: convergência:	Dados dos trech	E esultados da si 18-01-18- 18-01-18- 18-01-18- 18-01-18- 125 13 0.000027 Sem problemas	s longo 🕀 F mulação hidrá 15-04-16 16-03-03 16-11-12	G G G Caminh Núme Número de in Fraturas con Face de volume Face Esquerda	H o mais longo ro de fraturas gera ro de interseções ol terseções no camini a condição de contor ra face do volume 100	idas: titdas: no mais longo: ino: Cargas hidrául ID 11	57 40 15 Valor na fratura (m) 95.1478_	ĸ	
> Dado	S das fraturas C B Data e hora de ge Data e hora da sis Data e hora de expo Volume de s Volume de s túmero de iteraçõe Erro relati Situação de	c c ação da geometria: mulação do idráulica: rtação dos resultado imulação (m ³): do cálculo hidráulic vo residual: convergência:	Dados dos trech D FratCond - Re	Caminho mai E esultados da si 18-01-18- 18-01-18- 18-01-18- 125 125 13 0.000027 Sem problemas	F F mulação hidrá 15-04-16 16-03-03 16-11-12	G G Iulica - Caminh Núme Número de in Face do volume Face do volume Face Squerda	H o mais longo ro de interseções ol terseções no camini n condição de contor r na face do volume 100 90	i adas: ttidas: no mais longo: tro: Cargas hidráu ID 11 27	J 57 40 15 Valor na fratura (m) 96.1478 90.4819	K	
Dado A	B B Data e hora de ge Data e hora de se Data e hora de expo Volume de s Volume de iteraçõe Erro relati Situação de	c c ação da geometria: nulação hidráulica: tação dos resultado tação (m [*]): inulação (m [*]): do cálculo hidráulis convergência:	Dados dos trech	E E E E E E E E E E E E E E E E E E E	F F mulação hidrá 15-04-16 16-03-03 16-11-12	G G iulica - Caminh Núme Número de in Fraturas con Face Esquerda Face Direita Face Frontal	H o mais longo no de interseções ol terseções o de interseções o terseções no camitor rona face do volume 100 90 85	i das: o mais longo: mo: Cargas hidrául 10 11 27 14	J 57 40 15 Valor na fratura (m) 96.1478 90.4815 85.3374	ĸ	
> Dado	S das fraturas C B Data e hora de ge Data e hora da sin Data e hora da sin Data e hora de expo Volume de s Volume de s túmero de iteraçõe Erro relati Situação de	c c ação da geometria: nulação hidráulica: rtação o nesultado imulação (m ¹): e do cálculo hidráulic vo residual: convergência:	Dados dos trech	E esultados da si 18-01-18- 18-01-18- 18-01-18- 18-01-18- 125 125 13 0.000027 Sem problemas	F F mulação hidrá 15-04-16 16-03-03 16-11-12	G G Iulica - Caminh Núme Número de in Fraturas con Face Esquerda Face Squerda Face Frontal Face Frontal	H o mais longo ro de interseções ol terseções no camini rondição de contor rna face do volume 100 90 85 90	idas: ttidas: no mais longo: mo: Cargas hidrául 10 11 27 14 20	J 57 40 15 Kalo Instratura (m) 96.1478 90.4819 85.3374 88.8512	ĸ	
A A E	S das fraturas C B Data e hora de ge Data e hora da sis Data e hora de expo Volume de s Volume de s Erro relati Situação de	c c ação da geometria: mulação hidráulica: rtação dos resultado imulação (m [*]): do cálculo hidráulic vo residual: convergência:	Dados dos trech D FratCond - Re	Caminho mai E esultados da si 18-01-18- 18-01-18- 18-01-18- 125 125 13 0.000027 Sem problemas	F F mulação hidrá 15-04-16 16-03-03 16-11-12	G iulica - Caminh Núme Númeo de in Fraturas con Face de squerda Face Frontal Face Frontal Face fundo	H o mais longo no de interseções ol terseções no camini nondição de contor r na face do volume 100 90 85 90	ndas: tidas: to mais longo: tidas: tidas: to mais longo: tidas:	J 57 40 15 Valor na fratura (m) 90.4478 90.4819 85.3374 89.8512	K	
A A I N	B B Data e hora de ge Data e hora de sin Data e hora de sin Data e hora de expo Volume de si Volume de iteraçõe Erro relati Situação de fratura i	c c ação da geometria: nulação hidráulica: rtação dos resultado imulação (m³): s do cálculo hidráulic vo residual: convergência: fratura j	Dados dos trech	Caminho mai E esultados da si 18-01-18- 18-01-18- 18-01-18- 125 13 0.000027 Sem problemas C (m²/5)	F F mulação hidrá 15-04-16 16-03-03 16-11-12 h - fratura I (m)	G G G G Caminh Núme Número de in Face A volume Face E volumero Face E volumero Face E volumero Face E volumero Face E volumero Face Face Fronta Face Fordura Face	H o mais longo o de interseções ol terseções no camint o condição de contou r na face do volume 100 90 85 90 1 (m/m)	vdas: tridas: no mais longo: no: Cargas hidrául 11 27 14 20 Q (m²/s)	J 57 40 15 Kaiorna fratura (m) 96.1478 90.4819 85.3374 89.8512	ĸ	
A A I N N	S das fraturas C B Data e hora de ge Data e hora de sin Data e hora de expo Volume de s Aúmero de iteraçõe Erro relati Situação de fratura i 2	c c ação da geometria: nulação dis resultado imulação (m*): s do cálculo hidráulič vo residual: convergência: fratura j 36	Dados dos trech D FratCond - Re s: o: 	E E E E E E E E E E E E E E E E E E E	F F mulação hidrá 15-04-16 16-03-03 16-11-12 h - fratura i (m) 87.6384	G G G Carrinh Núme Número de in Frace Esquerda Face Direita Face Frontal Face Frontal Face Frontal Face Grundo h - fratura j (m) 89.0143	H o mais longo ro de interseções ol terseções no camini r condição de contor r na face do volume 100 90 85 90 1 (m/m) 0.5313	I Idas: titidas: no mais longo: ID 11 27 14 20 Q (m³/s) 0.0008	J 57 40 15 Valor na fratura (m) 96.1478 90.4819 85.3374 89.8512	κ	
) Dado A ID 5 6	B B Data e hora de ge Data e hora de sep Data e hora de expo Volume de sep Volume de iteraçõe Erro relati Situação de fratura i 2 2	c c ação da geometria: nulação hidráulica: tação dos resultado tação (m [*]): inulação (m [*]): do cálculo hidráuli convergência: fratura j 36 40	Dados dos trech D FratCond - Re 5: 0: 0: 1.5898 1.5318	Caminho mai E esultados da si 18-01-18- 18-01-18- 18-01-18- 125 125 13 0.000027 Sem problemas C (m³/s) 0.0015 0.0024	F F mulação hidrá 15-04-16 16-03-03 16-11-12 h - fratura i (m) 87.6384 87.6384	i (G iulica - Caminh Núme Número de in Face Dervisor Face Dervisor Face Dervisor Face Dervisor Face de fundo h- fratura j (m) 89.0143 87.5099	H o mais longo o mais longo o de interseções ol terseções no camini condição de contor r na face do volume 100 90 85 90 1(m/m) 0.5313 0.0839	I stdas: to mais longo: to mais longo: to to to to to to to to to to	J 57 40 15 Valor na fratura (m) 96.1478 90.4815 85.3374 89.8512	K	
) Dado A A ID ID 5 6 7	S das fraturas C B Data e hora de ge Data e hora de sin Data e hora de sin Data e hora de sin Data e hora de sin Volume de s Volume de s Erro relati Situação de fratura i 2 2 2	c c ação da geometria: nulação dos resultados imulação (m*): s do cálculo hidráulic vo residual: convergência: fratura j 36 40 56	Dados dos trech D FratCond - Re s: 0:	E esultados da si esultados da si 18-01-18- 18-01-18- 125 125 13 0.000027 Sem problemas C (m²/s) 0.0015 0.0024 0.0017	F F T15-04-16 15-04-16 16-03-03 16-11-12 h - fratura i (m) 87.6384 87.6384 87.6384	G G G S S S S S S S S S S S S S	H o mais longo o mais longo o de interseções ol terseções no camini torondição de contor ra face do volume 100 90 85 90 1 (m/m) 0.5313 0.0839 0.2732	I sdas: ttidas: no mais longo: ID 11 27 14 20 Q. (m ² /s) 0.0008 0.0002 0.0005	57 40 15 icas impostas Valor na fratura (m) 96.1478 90.4819 85.3374 89.8512	ĸ	
) Dado A I <	Si das fraturas C B Data e hora de ge Data e hora de sin Data e hora de expo Volume de s Volume de s Volume de iteraçõe Erro relati Situação de fratura i 2 2 2 10	c c ação da geometria: mulação dos resultado imulação (m [*]): s do cálculo hidráulic vo residual: convergência: fratura j 36 40 56 48	Dados dos trech D FratCond - Re : : : : : : : : : : : : : : : : : : :	Caminho mai E esultados da si 18-01-18- 18-01-18- 125 125 13 0.00027 Sem problemas C (m²/s) 0.0015 0.0024 0.0017 0.0017	F F mulação hidrá 15-04-16 16-03-03 16-11-12 h - fratura i (m) 87.6384 87.6384 87.6384 92.4546	G G S S S S S S S S S S S S S	H o mais longo o mais longo o de interseções ol terseções no camini nondição de contor r na face do volume 100 90 85 90 1 (m/m) 0.5313 0.0839 0.2732 2.2708	i das: titas: no mais longo: mo: Cargas hidrául 10 11 27 14 20 0.0008 0.0008 0.0008 0.0008 0.0005 0.0005	J 57 40 15 icas impostas Valor na fratura (m) 96.1478 90.4819 85.3374 89.8512	ĸ	

Figure 10 - Spreadsheet tabs with part of numerical results obtained by FratCond module 2

Source: Authors (2020)

4. CONCLUSIONS

Considering the results presented, we conclude that FratCond module 2 is able to take advantage of the results provided by module 1 and to simulate a fractured medium, considering the approach of equivalent one-dimensional pipes. The process to obtain pipes geometry and the calculation of hydraulic variables were well implemented, considering the methodology used in this study. The results analysis is facilitated using graphs and spreadsheets available to the user. We expect, in a possible study continuation, an additional module implementation, capable of evaluating the solutes transport in fractured media, considering 1D pipes approach; and possible simulations of fractured media with real data, for a full evaluation of FratCond tool potentialities.

ACKNOWLEDGMENTS

The first author is grateful to FAPEMIG (Minas Gerais State Agency for Research and Development) for its support, with a postgraduate scholarship.

Both authors also thank the National Council for Scientific and Technological Development (CNPq) for the financial support given to the project entitled "Construção de um Modelo Computacional Livre para Simulação de Escoamento e do Transporte de Poluentes em Aquíferos Utilizados no Abastecimento de Água" (Process: 446659 / 2014-2, MCTI/CNPQ/ Universal Notice 14/2014).

REFERENCES

BARENBLATT, G.E.; ZHELTOV, I.P.; KOCHINA, I.N. Basic concepts in the theory of homogeneous liquids in fisssured rocks. *Journal of Applied Mathematics and Mechanics*. USSR v. 24. n. 5. p. 1286–1303, 1960. https://doi.org/10.1016/0021-8928(60)90107-6

BODIN, J.; POREL, G.; DELAY, F.; UBERTOSI, F.; BERNARD, S.; DE DREUZY, J. Simulation and analysis of solute transport in 2D fracture/pipe networks: The SOLFRAC program. *Journal of Contaminant Hydrology*. v. 89. p.1–28, 2007. <u>https://doi.org/10.1016/j.jco-nhyd.2006.07.005</u>

CACAS, M. C.; LEDOUX, E.; MARSILY, G.; TILLIE, B.; BARBREAU, A.; DU-RAND, E.; FEUGA, B.; PEAUDECERF, P. Modeling fracture flow with a stochastic discrete fracture network: Calibration and validation. 1. The flow model, *Water Resources Research*. v. 26. n. 3. p. 479-489, 1990a. <u>https://doi.org/10.1029/WR026i003p00479</u>

CACAS, M. C.; LEDOUX, E.; MARSILY, G.; BARBREAU, A; CALMELS, P.; GAILLARD, B.; MARGRITTA, R. . Modeling fracture flow with a stochastic discrete fracture network: Calibration and validation. 2. The transport model. *Water Resources Research.* v. 26. n. 3. p. 491-500, 1990b. <u>https://doi.org/10.1029/WR026i003p00491</u>

CHILÈS, J. P. Stochastic modeling of natural fractured media: a review. In: LEUANGTHONG, O., DEUTSCH, C.V. (EDS.). GEOSTATISTICS BANFF OF QUANTITATIVE GEOLOGY AND GEOSTATISTICS, 14., 2005. [*Proceedings...*]Springer: Netherlands. p. 285–294, 2005. https://doi.org/10.1007/978-1-4020-3610-1_29

DERSHOWITZ, W. S. Rock mechanics approaches for understanding flow and transport pathways. In: EUROCK '96 ISRM - INTERNATIONAL SYMPOSIUM ON PREDICTION AND PERFORMANCE IN ROCK MECHAN-ICS AND ROCK ENGINEERING. [*Proceedings..*]. International Society of Rock Mechanics. Torino, Italy, 1996. DERSHOWITZ, W., POINTE, P.L., DOE, T.W. Advances in discrete fracture network modeling. In: US EPA/NGWA FRACTURED ROCK CONFER-ENCE. [*Proceedings.*] Portland, Maine, USA, 2004.

GRINGARTEN, A. C. Flow-test evaluation of fractured reservoirs. *Recent Trends in Hydrogeology* - Geol. Soc. Am. Special Paper, n.189. p. 237–263, 1982. <u>https://doi.org/10.1130/SPE189-p237</u>

JING, L. A review of techniques, advances and outstanding issues in numerical modelling for rock mechanics and rock engineering. *Int. J. Rock Mech. Min. Sci.* v. 40. n. 3. p. 283–353, 2003. https://doi.org/10.1016/S1365-1609(03)00013-3

MORENO, L.; NERETNIEKS, I. Fluid flow and solute transport in a network of channels. *Journal of Contaminant Hydrology*. v. 14, p. 163-192, 1993. <u>https://doi.org/10.1016/0169-7722(93)90023-L</u>

OUTTERS, N.; SHUTTLE, D. Sensitivity analysis of a discrete fracture network model for performance assessment of Aberg. Golder Associates, Sweden. 68 p. 2000. Relatório.

PAIGE, C; SAUNDERS, M. LSQR: An Algorithm for Sparse Linear Equations and Sparse Least Squares. *ACM Transactions on Mathematical Software*, v. 8, n. 1, p. 43-71. 1982. https://doi.org/10.1145/355984.355989

REIS, A.; ALAMY FILHO, J. E. Desenvolvimento de um gerador estocástico de fraturas para modelagem de aquíferos fraturados -

módulo I da ferramenta Fratcond. Águas Subterrâneas, v. 32, n. 2, p. 181-190, 2018.

SEGAN, S.; KARASAKI, K. *TRINET:* a flow and transport code for fracture networks—user's manual and tutorial - LBL-34834. Lawrence Berkeley Laboratory, Berkeley, Califórnia. 1993.

SINGHAL, B. B. S.; GUPTA, R. P. *Applied Hydrogeology of Fractured Rocks* – Second Edition. New York: Springer, 2010. https://doi.org/10.1007/978-90-481-8799-7

SNOW, D.T. A Parallel Plate Model of Fractured Permeable Media. 1965. PhD Dissertation, University of California. 1965.

STRELTSOVA, T. D. Hydrodynamics of groundwater flow in a fractured formation. *Water Resources Research.* v. 12, n. 3, p. 405-414, 1976. https://doi.org/10.1029/WR012i003p00405

UBERTOSI, F.; DELAY, F.; BODIN, J.; POREL, G. A new method for generating a pipe network to handle channelled flow in fractured rocks. *C. R.* Geoscience. n. 339, p. 682–691, 2007. https://doi.org/10.1016/j.crte.2007.07.013

XU, C.; FIDELIBUS C.; DOWD, P. A. Realistic pipe models for flow modelling in Discrete Fracture Networks. In: INTERNATIONAL DISCRETE FRACTURE NETWORK ENGINEERING CONFERENCE. [Proceedings...] 2014.